MARION COUNTY COMMISSION
MARION COUNTY, WEST VIRGINIA

PALATINE PARK IMPROVEMENTS

ADDENDUM #1

MARCH 21, 2022

THRASHER PROJECT #030-10387

TO WHOM IT MAY CONCERN:

A Pre-Bid Conference was held on Wednesday, March 16, 2022, on the above-referenced project, a copy of the sign in sheet is included in this Addendum. The following are clarifications and responses to questions posed by contractors for the above reference project.

A. GENERAL

1. Note about "Galvanizing on Sheet S001 is to be removed. All exposed steel shall be painted and not galvanized.

B. DRAWINGS

Revisions made to C001, C002 and S001 as noted.

C. QUESTIONS AND RESPONSES

QUESTION

1. There is a note on C002 – "All existing light fixtures shall be upgraded/replaced with a high lumen output option" Is this part of the scope of work? If so, how many lights and can you get us a spec on these?

RESPONSE

This note has been removed. There is no intended work on the existing park lights as part of this contract.

QUESTION

2. Is there soffit on the bottom of the deck – eave/rake trim detail looks like there is soffit.

RESPONSE

No there is no soffit intend on this project. The bent metal plate supports the edge of the roof decking, see 5/S500. The remainder of the ceiling is exposed metal roof decking.

QUESTION

3. Is there a place onsite to get rid of footing spoils or do we need to haul off site?

RESPONSE

The foundation spoil can be wasted in the area between the parking lot and the river. Material will need to be spread and graded to provide positive drainage and will need to be seeded.

QUESTION

4. The length is more of the issue with all this material. We don't have the ability to handle 60 ft material in the shop in Morgantown where it would have to be fabricated to try to meet the schedule you provided. Also, the cost will be quite a bit more substantial because all the material will need to be double dipped at the galvanizer as well as over length on delivery. Would it be possible for the engineer to provide a splice detail that would be acceptable?

RESPONSE

A detail has been added to Sheet S500 for a splice if necessary. This splice is applicable along the entire length of the beam.

If you have any questions or comments, please feel free to contact me at your earliest convenience. As a reminder, bids will be received until 2:00 p.m. on Wednesday, March 23, 2022, at Marion County Commission, 200 Jackson Street, Room 403, Fairmont, WV. Good luck to everyone and thank you for your interest in the project.

Sincerely,

THE THRASHER GROUP, INC.

Richard Hovatter, PE
Project Manager

RICHARD G. HOVATTER JR. REGISTERED 18460 STATE OF WEST VIRGINIA PROFESSIONAL ENGINEER
3/21/22

MARION COUNTY COMMISSION
MARION COUNTY, WEST VIRGINIA
PALATINE PARK IMPROVEMENTS

PRE-BID CONFERENCE
Wednesday, March 16, 2022

Thrasher Project #030-10387

Name	Representing	Phone #	Email Address
Jeremy P Monday	Maternicola's Masonry LLC	304 629 6767	mondayjeremy@gmail.com
Sheldon Raber	Lee Roger Builds	304-592-2083	sraber@LRBUILDSWV.com
Jason McQuain	City Construction	304-623-2573	Jason@CCCWV.US
Scott Willis	Veritas Contracting	304-641-9615	SWILLIS@VERITASWV.COM

PLAN LEGEND

- EXISTING PROPERTY/ RW LINE
- EXISTING CONTOURS
- EXISTING GRAVEL ROAD
- EXISTING PAVED ROAD
- EXISTING STRUCTURE
- EXISTING CULVERT
- EXISTING BIO-RETENTION
- PROPOSED FILL LIMIT
- PROPOSED CUT LIMIT

NORTH

20 FT 0 20 FT 40 FT

PLOT DATE/TIME: 3/17/2022 2:15 PM

LAYOUT TAB: C001

CAD FILE: R:\030\030-10387.00-PALATINE PARK IMPROVEMENTS-MARION CO COMMISSION-\Drawing\10387-Park Canopy Site Plan Rev3-17-22.dwg

NO.	BY	DATE	DESCRIPTION
1	RJH	3/17/22	REMOVED NOTE ABOUT PARK LIGHTS, IDENTIFIED LOCATION OF SOIL WASTE

Marion County
WEST VIRGINIA

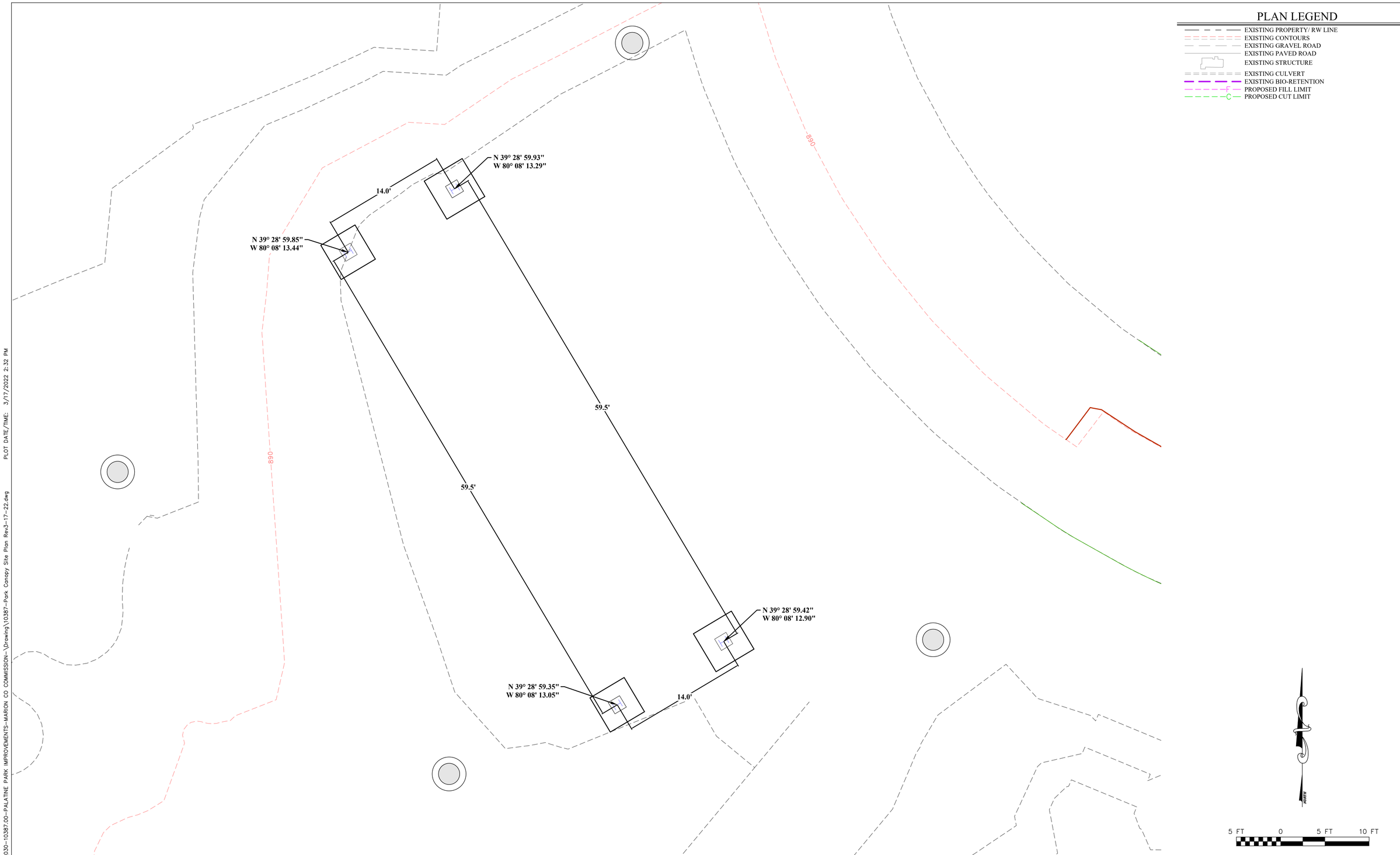
SCALE: AS SHOWN	
DRAWN:	DATE:
CHECKED:	DATE:
APPROVED:	DATE:
SURVEY DATE:	
SURVEY BY:	
FIELD BOOK No.:	

THRASHER
THE THRASHER GROUP, INC.
600 WHITE OAKS BLVD, BRIDGEPORT, WV 26330
PHONE (304) 624-4108
FAX (304) 624-7831

PHASE No.	
CONTRACT No.	
PROJECT No.	030-10387

PALATINE PARK IMPROVEMENTS
FAIRMONT, WEST VIRGINIA
MARION COUNTY
OVARALL SITE PLAN

SHEET No.
C001



LAYOUT TAB: C002
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PLOT DATE/TIME: 3/17/2022 2:32 PM

NO.	BY	DATE	DESCRIPTION
1	RJH	3/17/22	REMOVED NOTE ABOUT PARK LIGHTS, IDENTIFIED LOCATION OF SOIL WASTE

Marion County
WEST VIRGINIA

SCALE: AS SHOWN	
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APPROVED:	DATE:
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SURVEY BY:	
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PHASE No.

CONTRACT No.

PROJECT No.
030-10387

PALATINE PARK IMPROVEMENTS
FAIRMONT, WEST VIRGINIA
MARION COUNTY
DETAILED SITE PLAN

SHEET No.
C002

GENERAL STRUCTURAL NOTES:

1. Design Code: IBC 2015, Risk Category II

2. No provisions have been made for future horizontal or vertical expansion.

3. Wind Design Data:

   - Ultimate 3-Second Gust Design Wind Speed: 115 mph
   - Nominal 3-Second Gust Design Wind Speed: 74 mph
   - Wind exposure classification: C

   A. Internal Pressure Coefficient: 0.85
   Components and Cladding
   B. Design wind pressure for exterior components: 19 psf

4. Earthquake Design Data:

   - Seismic Importance Factor, I_E = 1.00
   - Mapped spectral response acceleration parameters:
     S_S =0.109, S_1 = 0.056
   - Design spectral response acceleration parameters:
     S_{DS} =0.116, S_{D1} = 0.089
   - Seismic Site Class: D
   - Seismic Design Category: B
   - Basic Seismic Force Resisting System:
     Ordinary Moment Frames
   - Design Base Shear: 2 kips
   - Seismic Response Coefficient, Cs: 0.921
   - Response Modification Coefficient, R: 3.0
   - Analysis Procedure Used:
     Equivalent lateral force procedure

5. Snow load data:

   - Ground Snow Load, P_g = 25 psf
   - Flat Roof Snow Load, P_f = 23 psf (min)
   - Snow Exposure Factor, C_e = 0.9
   - Thermal Factor, C_t = 1.2
   - Snow Load Importance Factor, I_s = 1.0

6. General Contractor shall verify all dimensions and conditions related to existing construction, existing services, and the site.

7. Construction loads shall not exceed design live loads. Shoring and re-shoring is the responsibility of the General Contractor.

CAST-IN-PLACE CONCRETE:

1. The minimum ultimate compressive strength of concrete at 28 days shall be:

   a. 4000 psi

2. Air Content: All concrete exposed to freezing and thawing and/or required to be watertight shall have an air content of 4.5% to 7.5%. All other concrete shall have an air content of 3% to 4%.

3. Water Cement Ratio: All concrete subjected to exposed to freezing and thawing in moist condition and/or required to be watertight shall have a maximum water-cement ratio of 0.45. All reinforced concrete exposed to deicing salts, brackish water seawater or spray from these sources, shall have a maximum water-cement ratio of 0.40.

4. Maximum aggregate size shall be 1 1/2 ", well graded, well-shaped (not elongated, flat, or slippery), and free of clay, dirt, and excess fines, U.N.O.

5. Aggregate composition shall consist of quartz, limestone, dolomite, granite, or feldspar.

6. Cement shall be Type 1, U.N.O.

7. Maximum concrete slump 3", U.N.O.

8. Reinforcing bars: ASTM A615, Grade 60.

9. Welded wire fabric: ASTM A185.

10. Provide 6x6-w2.9xw2.9 welded wire fabric in all non-structural slabs on grade, unless otherwise noted.

11. Place reinforcement in slabs, 1-1/2" down from top of slab, unless otherwise noted.

12. Provide control joints in all non-structural slabs on grade. The maximum spacing of control joints shall be 20'-0" O.C. unless otherwise noted. Control joint depth equal to 1/5 slab thickness not less than 1 inch.

13. Reinforcing bar lap splices and anchorage lengths shall conform with ACI 318-11 "Development and Splices of Reinforcement." All splices shall be Type B.

14. Top layer of reinforcing steel in slabs and footings shall be considered top bars regardless of thickness of concrete below the bars.

15. All horizontal wall bars shall be bent lapped around all corners, unless otherwise noted.

16. Provide vertical and horizontal reinforcing bars in concrete walls to conform to the minimum provisions of ACI 318, Section 14.3 unless otherwise noted.

17. Chamfer exposed edges of concrete 1/2" unless otherwise noted.

18. Refer to architectural drawings for location and extent of finishes or other treatments to exposed concrete.

19. Determine size, location and weight of mechanical equipment and make provisions for bolts, sleeves, pads, etc. from manufacturer's certified drawings. This work shall be coordinated with the trades involved.

20. All new concrete shall be bonded to previously placed concrete per specification requirements, U.N.O.

21. The Contractor shall prepare shop drawings showing detail layouts of reinforcing, including dimensions, openings, and spacing, bending details, bar schedules, and similar items required for the proper construction of the work.Provisions for the connection of work by other trades shall be indicated on the shop drawings. The location of all embedded items shall be indicated by the contractor on the shop drawings. All shop drawings shall be submitted for approval in accordance with the requirements of the Contract Documents.

22. Preparing, curing, transporting, and testing concrete cylinders. For each class of concrete placed, at least four cylinders shall be taken for each 50 cubic yards, or fraction thereof, of each class of concrete placed each day. Cylinders are to be taken in accordance with ASTM C31 and results shall be submitted to the Architect/Engineer, Construction Manager and owner. Two cylinders will be tested at 7 days and two at 28 days.

FOUNDATIONS:

1. Foundation design is based on the recommendations from the geotechnical report prepared by NGE Environmental & Geotechnical Engineering Soutions, report number W21143 dated December, 2021. Geotechnical Report shall be a part of the bid documents and shall govern over any conflicting information on other documents. Structural engineer is not responsible for subsurface conditions encountered in the field different from those assumed for design.

2. For spread footings where the soil or rock capable of supporting the minimum specified geotech report bearing capacity is below the bottom of the footing elevation, based on the top of footing elevation shown on the drawings, the top of the footing elevation shall be maintained, and a plain concrete pad, consisting of concrete with a 28-day compressive strength of 3000 PSI and utilizing type I cement, shall be placed under the footing. The plansize of the plain concrete pad shall be at least as large as the spread footing. Foundations for miscellaneous foundation walls, cantilevered retaining walls, and miscellaneous structures shall consist of individual and continuous spread footings and shall bear on soil capable of supporting 1500 PSF.

3. Notify the Architect of any unusual soil conditions that are in variance with the test borings, such as ground water, substandard bearing material, or obstructions.

4. Refer to Foundation Plan for footing elevations. Elevations shown are "top of concrete".

5. Backfilling against foundation or pit walls will not be permitted until supporting floors at the top of these walls are in place and able to provide full support to the imposed loads. Proper temporary bracing may be used in lieu thereof with prior approval of the Architect. The design of the temporary bracing is the responsibility of the General Contractor. G.C.'s Shoring Design Engineer shall provide a certified design for approval. Shoring forces will be provided by the E.O.R. upon request.

6. The General Contractor shall be responsible for the design, installation, and final clearance of any required shoring or bracing.

7. Remove all unsuitable fill and replace per the recommendations of the Geotechnical Engineer of record.

8. Anchor bolts shall be set in place prior to concrete placement. They shall not be forced into wet concrete.

9. Unless noted otherwise in the geotechnical report or specifications, compact all fill under slabs on ground and foundations to 98% of optimum laboratory density in accordance with ASTM D698 Standard Proctor Method. Place fill in 6" to 8" lifts and compact with vibratory tamping equipment.

10. Locate existing underground utilities in areas of construction. Contact local authorities for coordination.

11. When excavations approach the ground water level, the water level shall be continuously lowered by an acceptable dewatering system so that the water level is maintained continuously a minimum of 2'-0" below the excavation.

STRUCTURAL STEEL:

1. All structural steel work shall be in accordance with the "Specifications for the Design, Fabrication, and Erection of Structural Steel Buildings" (14th Edition) of the AISC. Maintain copy of each on job site during construction.

2. Structural steel shall conform to the following:

   a. Wide flange shapes and WT's - ASTM A992 with a minimum yield strength of 50,000 PSI.

   b. Channels, angles, plates, and miscellaneous connection material - ASTM A36 with a minimum yield strength of 36,000 PSI unless noted otherwise.

   c. Pipes - ASTM A501 with a minimum yield strength of 36,000 PSI or ASTM A53 Type E or S with a minimum yield strength of 35,000 PSI.

   d. Tubes - ASTM A500, Grade B with a minimum yield strength of 46,000 PSI.

3. All bolts shall be 3/4" dia. unless noted otherwise ASTM A325 H.S. bolt of either friction or bearing type. Use slip critical connections for all wind bracing connections. Threads shall be included in the shear plane.

4. All bolted connections shall be made according to AISC Table II or III framed beam connections. The minimum depth of connection must be more than one half the depth of the beam except that beams framing to columns shall have full depth connections using 3/8" connection angles or plates. Contractor shall provide certified design for all shear connections by a Professional Engineer in the state in which the project is located. Submit calculations for moment connections using braced member capacity U.N.O. on plans. Minimum end reaction of beams:

   - W8's = 10 kips
   - W10's= 12 kips
   - W12's= 16 kips
   - W14's= 18 kips
   - W16's= 20 kips
   - W18's= 22 kips
   - W21's= 24 kips
   - W24's= 26 kips

5. All welding shall be in strict accordance with the standards of the AWS and the AISC. Use E70XX electrodes.

6. Do not paint steel where encased in concrete or at field weld areas.

7. No shop or field holes or cuts are to be placed in structural members unless indicated on the contract or shop drawings.

8. The Structural Steel Fabricator shall field verify all dimensions prior to fabrication. Particularly for stairs, handrail systems, etc.

9. The Structural Steel Fabricator shall provide for vertical and horizontal adjustment of all support assemblies.

10. The Structural Steel Fabricator and/or the General Contractor shall verify all existing dimensions and conditions at the site. All discrepancies found shall be reported to the Architect prior to preparation of shop drawings. Shop drawings shall include all field measurements and conditions.

11. Expansion bolts:Use expansive anchors of the diameter indicated on the drawings as manufactured by HILTI Fastening Systems or approved equal.

    a. In concrete, use HSL Heavy Duty Anchors.

    b. In brick and CMU, use sleeve and fill CMU cells at all bolt locations.

12. Anchor bolts must meet ASTM A1554 gr. 36 specifications and be 3/4" diameter (unless otherwise indicated).

13. All galvanizing shall be per ASTM A123 and A780. All steel exposed to the elements and masonry support members shall be galvanized. Backup steel supporting masonry veneer and precast support angles shall be zinc primed and painted.

14. Steel may be coated with Zinc Rich Primer in lieu of galvanizing. Primer shall comply with VOC limitations of authorities having jurisdiction.

ALL STEEL SHALL BE PAINTED

REMOVE NOTE ABOUT GALVANIZING

15. Refer to architectural and mechanical drawings for possible miscellaneous steel. This steel shall also conform to the requirements in these General Notes and the Structural Steel specifications.

16. Steel fabricator shall review Architectural drawings and include all miscellaneous steel in their bid. If notes on architectural drawings refer to "see structural" and the structural drawings do not address this item notify the E.O.R. at least two weeks prior to bid opening to allow time for issue of addendum.

17. Column Schedule may not include all columns on the project. Review all drawings to insure all columns are included in bid.

18. Steel lintels can be replaced with engineered reinforced precast lintels. Mfgr shall submit certified Engineer design with lintel submittal.

SHOP DRAWINGS:

1. See Project Manual - Administrative Requirements for submittal procedures.

2. The General Contractor shall review, check, and stamp "Approved" all shop drawings prior to submitting them to the Architect. Shop drawings which have not been stamped "Approved" by the General Contractor do not conform to the requirements of the Contract Documents and will be rejected.

3. The General Contractor shall provide a shop drawing submittal schedule to the Architect at least two weeks prior to submittal of the first set of shop drawings.

4. Shop drawing submittals and review must precede the start of fabrication. General Contractor proceeds at their own risk without reviewed as noted shop drawings from the Architect. The maximum turn-around time for shop drawings will be two weeks (ten working days) from the date of receipt in the Architect's office to the date of return delivery. General Contractor and Architect shall increase coordination efforts when large numbers of shop drawings are invited.

5. Refer to the Project Manual (if available) for specific submittal procedures.

6. Shop drawings shall not simply be a copy of the engineer's design drawings with the contractor's title block. Electronic files from the E.O.R. can be provided to the G.C. at a negotiated fee. Fee will be based on amount of subcontractor's contract and benefit to subcontractor.

7. Shop drawings shall be completely prepared by the submitting entity.

STRUCTURAL SYMBOLS AND HATCHING	
SYMBOL	EXPLANATION
	MOMENT CONNECTION
	CANTILEVER MOMENT CONNECTION
	DIRECTION OF DECK SPAN
	BEAM BEARING PLATE
	TEFLON BEAM BEARING PLATE
	SPOT ELEVATIONS
	EXPANSION JOINT
	CONCRETE WALL
	MASONRY WALL
	AREA TO RECEIVE PAVERS

DRAWING LIST	
DRAWING NUMBER	DRAWING NAME
S001	GENERAL NOTES
S100	FLOOR PLAN
S300	SECTIONS
S500	DETAILS

FOR CONSTRUCTION

NO.	BY	DATE	DESCRIPTION

Allegheny Design Services
Consulting Engineers
102 Leeway Street, Morgantown, WV 26505
Phone: 304.599.0771

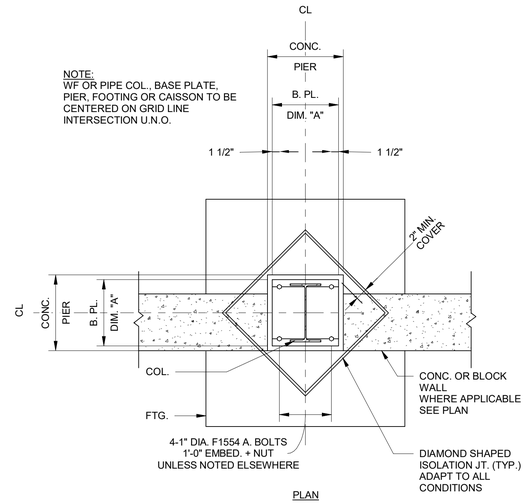
SCALE: AS SHOWN
DRAWN: BRH DATE: 3-2-22
CHECKED: DRS DATE: 3-2-22
APPROVED: DRS DATE: 3-2-22
SURVEY DATE:
SURVEY BY:
FIELD BOOK No.:

THRASHER
THE THRASHER GROUP, INC.
600 WHITE OAKS BLVD, BRIDGEPORT, WV 26330
PHONE (304) 624-4108
FAX (304) 624-7831

PHASE No.
CONTRACT No.
PROJECT No. 030-10387

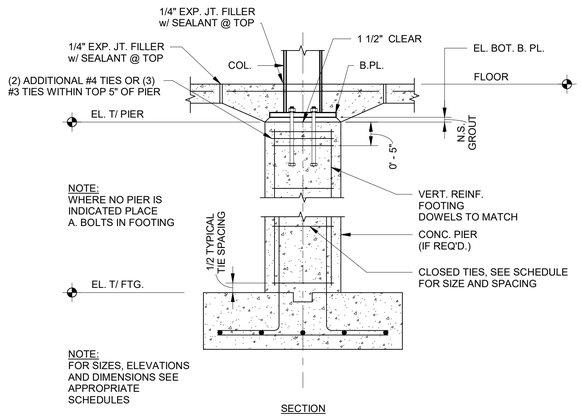
PALATINE PARK
IMPROVEMENTS
FAIRMONT, WEST VIRGINIA
MARION COUNTY

SHEET No. S001



NOTE:
WF OR PIPE COL., BASE PLATE, PIER, FOOTING OR CAISSON TO BE CENTERED ON GRID LINE INTERSECTION U.N.O.

PLAN

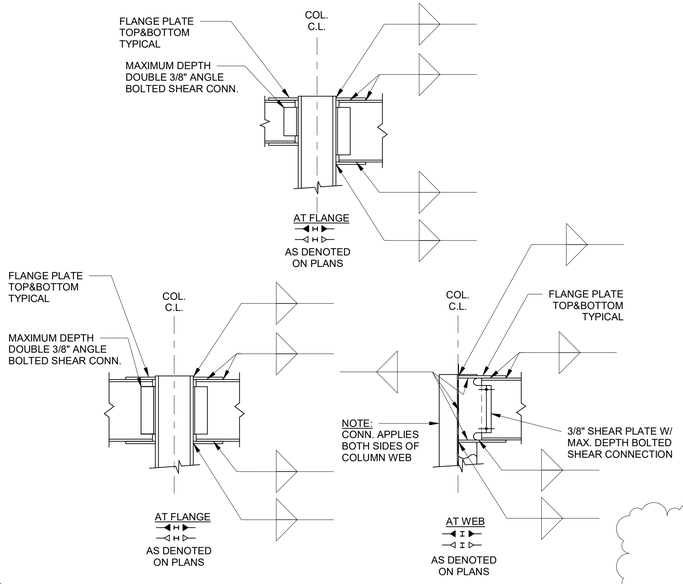


NOTE:
WHERE NO PIER IS INDICATED PLACE A. BOLTS IN FOOTING

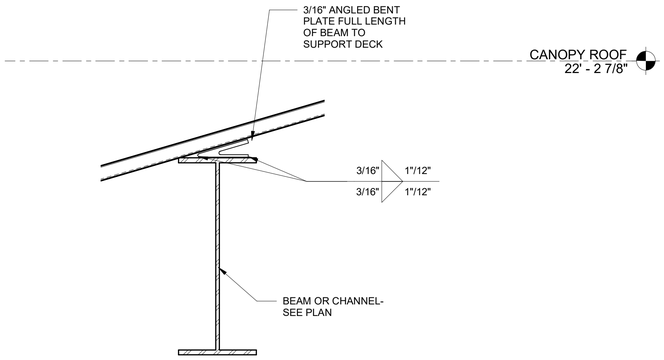
NOTE:
FOR SIZES, ELEVATIONS AND DIMENSIONS SEE APPROPRIATE SCHEDULES

SECTION

1 / S500 TYPICAL COLUMN BASE AT SPREAD FOOTING DETAIL

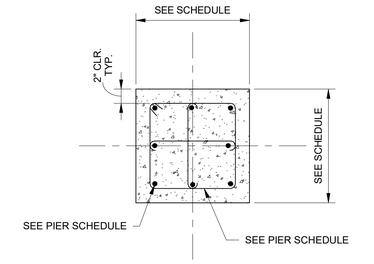


3 / S500 TYPICAL BEAM TO COLUMN MOMENT WELDED FLANGES

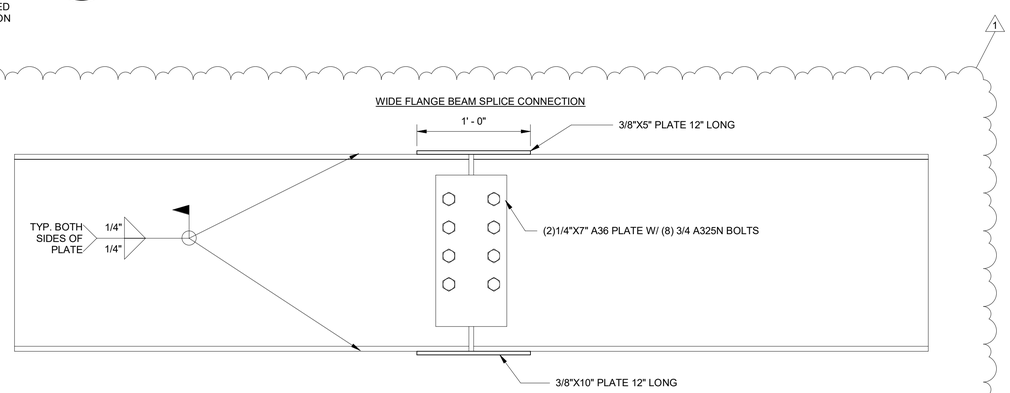


6 / S500 ANGLED BENT PLATE ON BEAM DETAIL

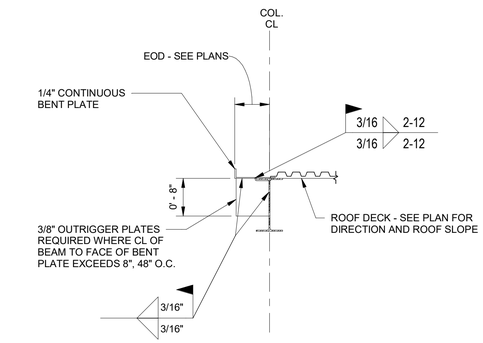
SCALE: 1 1/2" = 1'-0"



4 / S500 TYPICAL CONCRETE PIER DETAIL

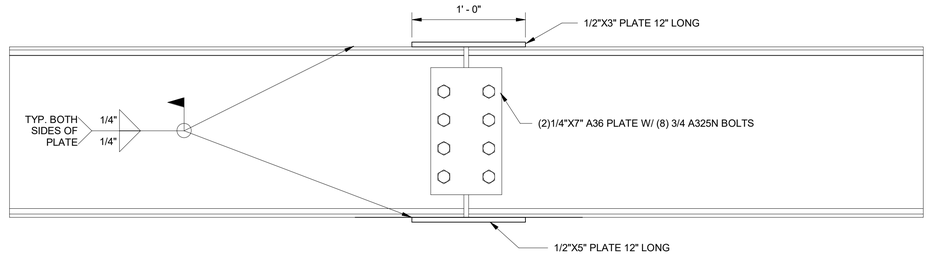


2 / S500 TYPICAL BEAM BEARING ON STEEL COLUMN DETAIL



5 / S500 TYPICAL SECTION AT ROOF

MC BEAM SPLICE CONNECTION



7 / S500 TYPICAL BEAM SPLICE CONNECTION DETAIL

SCALE: 1 1/2" = 1'-0"

FOR CONSTRUCTION

NO.	BY	DATE	DESCRIPTION
1	BRH	3/17/22	BEAM SPLICE DETAIL

Allegheny Design Services
Consulting Engineers
102 Leeway Street, Morgantown, WV 26505
Phone: 304.599.0771

SCALE: AS SHOWN	
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